PRECAST CONCRETE SWIMMING POOL

GENERAL NOTES

1. THE POOL INSTALLER SHALL BE RESPONSIBLE FOR MAINTAINING STABILITY OF THE STRUCTURE UNTIL COMPLETION OF CONSTRUCTION AND SHALL ENSURE THAT NO PART OF THE STRUCTURE IS OVER STRESSED BY EXCESSIVE CONSTRUCTION LOADING.
2. TEMPORARY WORKS ARE THE RESPONSIBILITY OF THE POOL INSTALLER, THESE INCLUDE SUCH ITEMS AS TEMPORARY SHORING & RETENTION, MAINTAINING TEMPORARY STABILITY OF THE STRUCTURE, FORMWORK, CRANE BASE, TEMPORARY WORKING PLATFORMS AND GROUND IMPROVEMENT TO SUPPORT CONSTRUCTION PLANT.
3. STRUCTURAL DRAWINGS TO BE READ IN CONJUNCTION WITH ALL POOL INSTALLER AND OTHER CONSULTANTS DRAWINGS AND SPECIFICATION.
4. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE FOLLOWING CODES EXCEPT WHERE VARIED BY THE DRAWINGS:

AS1170.0-2002	STRUCTURAL DESIGN ACTIONS – PART 0: GENERAL PRINCIPLES
AS1170.1-2002	STRUCTURAL DESIGN ACTIONS – PART 1: PERMANENT, IMPOSED AND OTHER ACTIONS
AS2783-1992	USE OF REINFORCED CONCRETE FOR SMALL SWIMMING POOLS
AS2870-2011	RESIDENTIAL SLABS AND FOOTINGS
AS3600-2018	CONCRETE STRUCTURES
AS4678-2002	EARTH-RETAINING STRUCTURES
	NATIONAL CODE OF CONSTRUCTION (NCC 2016)

5. THE CODES, NOTES AND PROCEDURES REFERRED TO AND SHOWN ON THESE DRAWINGS APPLY AT THE TIME OF THE DRAWING ISSUE AND FOR A PERIOD OF 5 YEARS MAXIMUM THEREAFTER. WITH CONTINUED USE OVER A LONG PERIOD, THE CODES, NOTES AND PROCEDURES MAY BE SUBJECT TO ALTERATIONS AND MODIFICATIONS TO THIS DRAWING MAY BE REQUIRED.
6. THE DETAILS ON THESE DRAWINGS ARE PROVIDED ON GOOD FAITH THAT GEOTECHNICAL INVESTIGATION IS CARRIED OUT PRIOR TO INSTALLATION OF THE POOL AND THE GEOTECHNICAL RECOMMENDATIONS AND REPORT ARE PROVIDED BY THE CUSTOMER TO AN INDEPENDENT CERTIFIED STRUCTURAL ENGINEER FOR VERIFICATION. BLIGH TANNER CARRIES NO RESPONSIBILITY FOR THE POOL SHELL IF NOT INSTALLED IN ACCORDANCE WITH GEOTECHNICAL RECOMMENDATIONS AND DESIGN ASSUMPTIONS OUTLINED IN THESE DRAWINGS.
7. THE DETAILS ON THESE DRAWING APPLY ONLY FOR THE STANDARD POOL SHELL SIZE NOTED ON THE DRAWINGS.
8. THESE DRAWINGS DETAIL THE STRUCTURAL DESIGN OF THE POOL SHELL ONLY. ALL OTHER CERTIFICATION AND BUILDING REGULATION REQUIREMENTS INCLUDING SKIMMER BOX INSTALLATION, POOL WATER TREATMENT, DRAINAGE REQUIREMENTS, POOL FENCING, ELECTRICAL AND EARTHING REQUIREMENTS, COUNCIL SUBMISSIONS AND APPROVALS, ETC ARE THE RESPONSIBILITY OF THE CUSTOMER.
9. POOL INSTALLER TO ENSURE ANY FOOTING OR ADJOINING STRUCTURES ARE NOT UNDERMINED BY POOL EXCAVATION. ANY UNDERPINNING WORKS REQUIRED SHALL BE SUBJECT TO DESIGN AND CERTIFICATION BY AN INDEPENDENT CERTIFIED STRUCTURAL ENGINEER. UNDERPINNING WORKS TO BE DONE BEFORE EXCAVATION.
10. CUSTOMER AND POOL INSTALLER TO MAKE THEMSELVES AWARE OF ANY IN-GROUND SERVICES AND AVOID ANY CLASHES WITH POOL EXCAVATION ZONE PRIOR TO EXCAVATION.

SALT WATER POOL

1. IF SALT CHLORINATOR USED, CONCENTRATION TO BE KEPT BELOW 8,000 P.P.M

DESIGN CRITERIA

1. ALL LOADINGS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170 AND AS2783.
2. THE STRUCTURAL COMPONENTS ON THESE DRAWINGS HAVE BEEN DESIGNED FOR THE FOLLOWING LOADINGS:
   a. WATER PRESSURE – POOL WATER AT LEVEL SHOWN ON DRAWINGS.
   b. GROUNDWATER PRESSURE – GROUNDWATER LEVEL TAKEN AT TOP OF POOL WALLS.
   c. LATERAL EARTH PRESSURES –REFER 'FOUNDATIONS' NOTES.
   d. LIVE LOADING = 0.6kN
   e. SURCHARGE LOADING = 5kPa
   f. INSTALLATION LOADS – AS SHOWN ON DRAWINGS.
3. THE GEOTECHNICAL SITE CONDITIONS ARE A CRUCIAL COMPONENT IN THE DESIGN AND INSTALLATION OF THE POOL SHELL. IT IS THE RESPONSIBILITY OF THE CUSTOMER IS TO CONFIRM THE GEOTECHNICAL PARAMETERS FOR EACH SITE. REFER TO "FOUNDATIONS" NOTES FOR MORE INFORMATION.
4. THE DESIGN THE DESIGN REQUIRES THAT GROUND WATER PRESSURE TO THE POOL WALLS AND FLOOR SLAB BE RELIEVED BY INSTALLATION OF HYDROSTATIC VALVE WITH MAIN DRAIN DETAIL.
5. THE POOL SHALL ONLY BE EMPTIED IN AN APPROVED MANNER UNDER PROPERLY CONTROLLED CONDITIONS. OWNER TO CONDUCT REGULAR (YEARLY) MAINTENANCE CHECKS OF THE DRAIN VALVE TO ENSURE THAT MAIN DRAIN VALVE DOES NOT BECOME BLOCKED. IT IS CRUCIAL THAT THE OWNER CHECKS THAT THE DRAIN VALVE IS NOT BLOCKED AND IS FULLY FUNCTIONING PRIOR TO EMPTYING POOL.
6. OWNER TO ENSURE POOL REMAINS FULL IN THE EVENT OF FLOODING OR GENERAL RISE IN WATER TABLE LEVEL.
7. THE DESIGN ASSUMES THAT THE SITE IS RELATIVELY FLAT WITH GROUND SLOPING AWAY FROM POOL.
8. IT IS IMPORTANT THAT THE SITE BE WELL DRAINED. THE GROUND AROUND THE STRUCTURES SHOULD SLOPE AWAY AT 1 IN 50 AND THEN FALL TO THE STORMWATER SYSTEM TO PREVENT PONDING OF WATER AGAINST OR NEAR THE SURFACE OF THE POOL.

FOUNDATIONS

1. THE CUSTOMER SHALL ENGAGE A CERTIFIED GEOTECHNICAL ENGINEER TO CARRY OUT SOIL TESTING AND PROVIDE ADVICE PRIOR TO COMMENCING WORKS ON SITE. THE AIM OF THE GEOTECHNICAL INVESTIGATION IS TO PROVIDE ADVICE ON THE FOLLOWING ASPECTS OF THE SITE:
   a. GENERAL SITE CONDITIONS, INCLUDING PHOTOS;
   b. SUBSURFACE CONDITIONS INCLUDING GROUNDWATER PRESENCE (AS LOCATED);
   c. RECOMMENDATIONS OF DEWATERING RECOMMENDATIONS DURING EXCAVATION;
   d. DEPTH TO SUITABLE BEARING STRATA;
   e. EARTHWORKS CONSTRUCTION RECOMMENDATIONS INCLUDING SITE EXCAVATABILITY, FILL PLACEMENT SPECIFICATION, SUBGRADE PREPARATION RECOMMENDATIONS AND COMPACTION PROCEDURES AND SPECIFICATION;
   f. PREDICTED CHARACTERISTIC SURFACE MOVEMENT, SOIL SUCTION ZONE AND SITE CLASSIFICATIONS BASED ON SITE REACTIVITY IN ACCORDANCE WITH AS2870, INCLUDING THE EFFECT OF TREES PRESENCE NEAR THE PROPOSED POOL LOCATION;
   g. SITE CONSTRAINTS AND CONSTRUCTION CONSIDERATIONS THAT MAY IMPACT SETOUT OF PROPOSED POOL LAYOUT, INCLUDING SLOPING SITES, EXISTING BUILDING FOUNDATIONS AND EXISTING TREES;
   h. RECOMMENDATIONS ON WHETHER THE EXCAVATED SOILS CAN BE USED AS STRUCTURAL FILL;
   i. MAINTENANCE PROGRAMMES FOR TREES, DRAINAGE, ETC
   j. MAXIMUM TEMPORARY AND PERMANENT BATTER SLOPES FOR BOTH FILL PLATFORMS AND CUTTINGS;
   k. LATERAL EARTH PRESSURES AGAINST THE WALLS;
   l. RECOMMENDATIONS ON FOOTING TYPES AND DESIGN PARAMETERS FOR HIGH LEVEL OR DEEP FOOTINGS;
   m. FIELDWORK COMPRISING OF MINIMUM 2 BOREHOLES (MINIMUM 6M DEPTH) TO BE UNDERTAKEN AT THE LOCATION OF WHERE THE POOL IS TO BE CONSTRUCTED;
   n. LABORATORY TESTING OF SOIL, GROUNDWATER AND ROCK TO PROVIDE DATA FOR GEOTECHNICAL PARAMETERS ASSESSMENT AS REQUIRED INCLUDING:
      i. MOISTURE CONTENT (MC);
      ii. ATTERBERG LIMITS; AND
      iii. PARTICLE SIZE DISTRIBUTION.
   o. POTENTIAL PRESENCE OF ACID SULPHATE SOILS; AND
   p. CONFIRM ALL GEOTECHNICAL ASSUMPTIONS USED IN DESIGN OF POOL SHELL:
      i. MAXIMUM UNIT WEIGHT OF SOIL = 19kN/m³
      ii. LONG TERM DRAINED f = 28° ('AT REST' EARTH PRESSURE COEFFICIENT, KO = 0.55)
      iii. MINIMUM ALLOWABLE BEARING PRESSURE = 100kPa.
2. THE SCOPE OF THE GEOTECHNICAL INVESTIGATION SHOULD ALSO NOTE THE FOLLOWING GENERAL REQUIREMENTS:
   a. THE FIELD WORK AND TESTING SHOULD BE CARRIED OUT UNDER THE SUPERVISION OF A QUALIFIED (RPEQ) GEOTECHNICAL ENGINEER, AND ONCE SAMPLES ARE RETURNED THE LABORATORY, THE LOGS PREPARED BY THE DRILLERS OPERATING THE DRILLING RIG ARE TO BE CHECKED AND VERIFIED .
   b. TESTING OF SAMPLES IS TO BE UNDERTAKEN IN AN NATA ACCREDITED LABORATORY. THE SCOPE OF THE SOIL INVESTIGATION SHOULD BE UNDERTAKEN WITH REFERENCE TO APPROPRIATE SAMPLING GUIDELINES AND AS1726.
3. THE GEOTECHNICAL CONSULTANT IS TO MAKE THEMSELVES AWARE OF ALL SERVICES AND EXISTING CONDITIONS IN THE AREA OF INVESTIGATION AND IS RESPONSIBLE FOR SAFETY OF PERSONNEL AND OTHERS ON SITE DURING SITE INVESTIGATION ACTIVITIES (I.E DIAL BEFORE YOU DIG, ETC).
4. THE STANDARD POOL SHELL DESIGN IS SUITABLE FOR CONSTRUCTION IN SITES OF CLASSIFICATION A, S AND M IN ACCORDANCE WITH AS2870. THIS STANDARD POOL SHELL DESIGN IS NOT SUITABLE FOR CONSTRUCTION IN SITES OF CLASSIFICATION E, H1, H2 OR P. WITHOUT REINFORCED BASE SLAB, GROUND IMPROVEMENT WORKS OR ADDITIONAL DEEP FOUNDATIONS.
5. FOUNDATIONS EXCAVATIONS TO BE MAINTAINED IN A FIRM DRY CONDITION. ALL UNCONTROLLED FILL, TOPSOIL AND ORGANIC MATTER UNDER POOL IS TO BE REMOVED.
6. EXCAVATION SHALL NOT BE PERFORMED BELOW THE LINE OF INFLUENCE EXTENDING FROM THE EXISTING FOOTINGS AS PROVIDED BY THE GEOTECHNICAL ENGINEER.
7. ALL FOUNDATIONS SHALL BE COMPACTED IN ACCORDANCE WITH GEOTECHNICAL REPORT RECOMMENDATIONS AND FOUNDED ON UNDISTURBED SOUND NATURAL GROUND CAPABLE OF WITHSTANDING A MINIMUM ALLOWABLE BEARING PRESSURE OF NOT LESS THAN 100kPa.
8. UNLESS OTHERWISE SPECIFIED BY THE GEOTECHNICAL ENGINEER, STRUCTURAL FILL TO BE NON-REACTIVE CBR15 PLACED IN 200 THICK LOOSE LAYERS AND COMPACTED TO 98% M.D.D.
9. WHERE THERE ARE ANY VARYING FOUNDATIONS OVER POOL AREA OR WHERE UNDISTURBED SOUND NATURAL GROUND IS NOT ENCOUNTERED, GEOTECHNICAL ENGINEER TO PROVIDE RECOMMENDATIONS ON GROUND IMPROVEMENT WORKS OR PROVIDE SUITABLE RELOCATION OF PROPOSED POOL AREA TO ACHIEVE COMPLIANCE WITH STRUCTURAL AND GEOTECHNICAL DESIGN CRITERIA.
10. IF OVER-EXCAVATION IS REQUIRED TO ACHIEVE UNDISTURBED SOUND NATURAL GROUND TO THE UNDERSIDE OF THE POOL BASE, THE OVER EXCAVATION SHALL BE BACKFILLED WITH 20mm SCREENING.
11. THE DESIGN OF THE POOL SHELL ASSUMES IMPERVIOUS BARRIERS (MASONRY PAVEMENTS OR 200mm THICK CLAY SEAL) OR BASE SLAB UNDER THE POOL IS INSTALLED IN ACCORDANCE WITH DETAILS SHOWN ON DRG S003. IT IS RECOMMENDED THAT A 2000mm WIDE RAMPED FULLY GRASSED SURROUNDING AREA FREE OF ANY CULTIVATION (PALMS AND 2m MAX SHRUBS PERMISSABLE). RAMPED SURROUND TO SURFACE DRAIN TO STORMWATER SYSTEM.
12. SIGNIFICANT TREES ARE TO BE SET BACK FROM EDGE OF POOL SHELL BY 1.0 TIMES THE MATURE TREE HEIGHT FOR SINGLE TREE, 1.5 TIMES THE MATURE TREE HEIGHT FOR A GROUP OF TREES AND 2.0 TIMES THE MATURE TREE HEIGHT FOR A GROUP OF 4 OR MORE TREES IN A ROW.
13. BACKFILL PLACED BEHIND THE WALL SHOULD BE LOOSE GRANULAR MATERIAL. ALL CONTROLLED BACKFILL BEHIND RETAINING WALLS IS TO BE LIGHTLY COMPACTED TO PREVENT EXCESSIVE COMPACTION-INDUCED LATERAL EARTH PRESSURES ON POOL WALLS. THE POOL BUILDER SHOULD USE LIGHT COMPACTION EQUIPMENT, WACKER PACKERS OR EQUIVALENT.

CONCRETE

C1 CONCRETE SPECIFICATION

SLUMP FLOW	650 +/- 40mm
MAXIMUM AGGREGATE	10 mm
CEMENT	GP

PROJECT CONTROL TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH AS.3600 AND THE SPECIFICATION TEST REPORTS TO BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.

C2 CONCRETE STRENGTH & CLEAR COVER TO BE AS FOLLOWS

ELEMENT	CONCRETE GRADE	COVER		
		BOTTOM	TOP	SIDES
PRECAST SHELL	S50	30	30	30
BASE SLAB	N32	50	50	50

C3 CURE ALL CONCRETE SURFACES IN ACCORDANCE WITH AS.3600.
CURING OF ALL CONCRETE IS TO BE ACHIEVED BY KEEPING SURFACES CONTINUOUSLY WET FOR A PERIOD OF 3 DAYS AND PREVENTION OF LOSS OF MOISTURE FOR A TOTAL OF 7 DAYS FOLLOWED BY A GRADUAL DRYING OUT. CURING COMPOUNDS NOT TO BE USED AS THEY MAY INHIBIT THE BOND OF SUBSEQUENT COATINGS.

C4 ALL RE-ENTRANT CORNERS AND PENETRATIONS LARGER THAN 200 DIAMETER ARE TO HAVE TRIMMER BARS PLACED AT CORNERS. TRIMMER BARS SHALL BE 1 N10 x 500 LONG UNO.

C5 ALL HOOKS AND BENDS TO BE IN ACCORDANCE WITH AS.3600.
UNLESS NOTED OTHERWISE ALL LAPS TO BE :

BAR SIZE	LAP (mm)
N10	250

C6 BASIC DRYING SHRINKAGE STRAIN MEASURED IN ACCORDANCE WITH AS.1012 - PART 13 SHALL NOT EXCEED 800 µm.

C7 CONDUITS, PIPES, ETC SHALL ONLY BE LOCATED IN THE MIDDLE ONE THIRD OF SLAB DEPTH AND SPACED AT NOT LESS THAN 3 DIAMETERS.

C8 REINFORCEMENT SYMBOLS
'N' - DENOTES GRADE 500 BARS TO AS.4671.
'R' - DENOTES GRADE 250 HOT ROLLED PLAIN BARS TO AS.4671.

C9 ALL CONCRETE TO BE VIBRATED DURING PLACEMENT.

C10 IT IS CRITICAL TO CONSTRUCT SLAB AND WALLS WITH CORRECT COVER TO REINFORCEMENT. REINFORCEMENT HAVE MINIMAL TOLERANCE AND ACCURATE SETOUT IS TO BE CONFIRMED BY SURVEY PRIOR TO CASTING.

C11 REINFORCEMENT SHALL BE ACCURATELY BENT ARROUND CURVATURES IN THE POOL PROFILE SHOWN ON THE DRAWINGS.

C12 THE REINFORCEMENT STEEL SUPPLIER MUST BE CERTIFIED BY ARCS (AUSTRALIAN STANDARDS CERTIFICATION & VERIFICATION OF REINFORCING, PRESTRESSING AND STRUCTURAL STEELS) FOR THE SUPPLY OF REINFORCEMENT STEEL.
REFER www.steelcertification.com FOR CURRENT CERTIFICATE HOLDERS.

C13 CONCRETE FINISH, TOLERANCES AND TONAL RANGE TO BE IN ACCORDANCE WITH CLASS 2 FINISH AS PER AS3610. ADDITIONAL FINISH REQUIREMENTS:
- NO CHIPS OVER 25mm DIAMETER
- NO AIR BUBBLES OVER 10mm DIAMETER
- NO HAIRLINE CRACKS OVER 100mm LONG

ABBREVIATIONS

ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION
HORIZ	HORIZONTAL	(o)	OVER
VERT	VERTICAL	(u)	UNDER
CENT	CENTRALLY PLACED	LG	LENGTH/LONG
CRS	CENTRES	w	WIDTH/WIDE
C/W	COMES WITH	h	HEIGHT/HIGH
B or BTM	BOTTOM FACE	d	DEPTH/DEEP
T	TOP FACE	NOM	NOMINAL
T&B	TOP & BOTTOM	REQD	REQUIRED
NF	NEAR FACE	REINF	REINFORCEMENT
FF	FAR FACE	OPP	OPPOSITE
EF	EACH FACE	SIM	SIMILAR
EW	EACH WAY	GA	GENERAL ARRANGEMENT
EQ	EQUAL	PT	POST TENSION
NSOP	NOT SHOWN ON PLAN	DRG	DRAWING
NSOE	NOT SHOWN ON ELEVATION	NTS	NOT TO SCALE
UNO	UNLESS NOTED OTHERWISE	FL	FLAT
TYP	TYPICAL	LL	LIVE LOAD
CL	CENTRE LINE	SDL	SUPERIMPOSED DEAD LOAD
PL	PLATE	THRU	THROUGH
CFW	CONTINUOUS FILLET WELD	NLB	NON LOAD BEARING
CPBW	COMPLETE PENETRATION BUTT WELD	MAX	MAXIMUM
		MIN	MINIMUM

PRECAST LIFTING AND INSTALLATION

1. PRECAST SHELL TO BE CAST WITH MINIMUM S50 CONCRETE. REFER CONCRETE NOTES.
2. REFER STRUCTURAL DRAWINGS FOR MINIMUM IN-SERVICE LOAD REINFORCEMENT. ALL DETAILS SHOWN ARE FOR WHEN PRECAST SHELL IS IN PLACE.
3. LIFTING INSERTS HAVE BEEN DESIGNED TO THE LOADS AS SHOWN ON THE DRAWINGS. ANY ADDITIONAL LIFTING STRESSES MUST BE CHECKED BY THE LIFTING CONTRACTOR PRIOR TO LIFTING.
4. LIFT PRECAST SHELL WITH SPREADER BAR AND PULLEYS AND LIFTING ROPES TO ENSURE EVEN LIFT & LOADING TO FERRULES. THE LIFTING CONTRACTOR MUST CHECK THE CAPACITY OF THE CAST IN FERRULES AGAINST THEIR TYPE OF LIFTING EQUIPMENT AND THE STRENGTH OF THE CONCRETE AT THE TIME OF LIFTING.
5. LIFTING LOADS ON PRECAST SHELL MUST ADHERE TO THE FOLLOWING:
   a. REMOVAL OF FORM AND LIFTING/ROTATING – FERRULES TO TAKE SHEAR LOADS ONLY. ROTATING OF PRECAST SHELL TO BE CARRIED OUT IN A SAFE AND CONTROLLED MANNER TO LIMIT WORKING SHEAR LOADS ON FERRULES AS SHOWN ON DRAWINGS.
   b. LIFTING/ROTATING: ROTATING SHELL BY MEANS OF A 'FLIPPER' IS TO BE CARRIED OUT IN A SAFE AND CONTROLLED MANNER. THE SHELL IS NOT TO BE SUPPORTED IN THE REGION NOTED ON THE GENERAL ARRAGEMENT PLAN.
   c. DURING INSTALLATION – FERRULES TO TAKE WORKING LOADS AS SHOWN ON DRAWINGS.
6. LIFTING CONTRACTOR TO ALLOW CONTROL TESTING OF CONCRETE SAMPLES TO CONFIRM MINIMUM CONCRETE STRENGTH AT TIME OF REMOVAL OF FORMS, LIFTING AND ROTATING AND INSTALLATION. MINIMUM CONCRETE STRENGTH FOR LIFTING TIMES ARE TO BE:
   a. REMOVAL OF FORMS AND LIFTING/ROTATING OF PRECAST SHELL: 20MPa
   b. DURING INSTALLATION: 50MPa
7. LIFTING CONTRACTOR TO ENSURE PRECAST SHELL IS STABLE AND SECURED PRIOR TO INSTALLATION.
8. CUSTOMER IS RESPONSIBLE FOR PROVIDING THE REQUIRED COUNCIL APPROVALS AND FORM 17 FINAL INSPECTION CERTIFICATE (OR EQUIVALENT IN EACH STATE) FOR THE PRECAST SHELL AND ENSURING THAT INSTALLATION COMPLIES WITH THE REQUIREMENTS SHOWN ON THE STRUCTURAL DRAWINGS.

BLIGH TANNER

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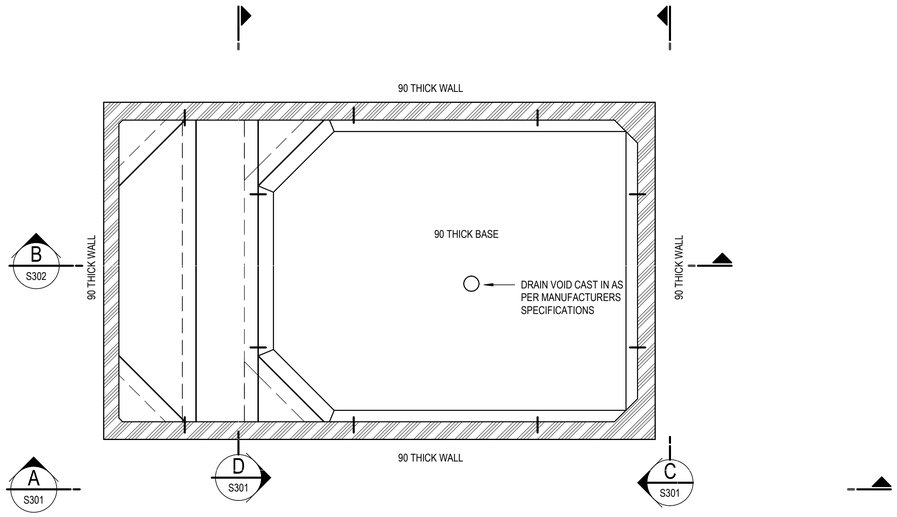
REV	DATE	DESCRIPTION	DESIGN	DRAWN	CHECKED	APPROVED	RPEQ No.
P1	17.07.2019	PRELIMINARY ISSUE	MW	RT			
C1	16.12.2019	CONSTRUCTION ISSUE	MW	MC			

PROJECT	PRECAST WATER HOLDING TANK	DRAWING TITLE	3.6m x 2.2m SHELL COVER AND GENERAL NOTES SHEET
LOCATION		ARCHITECT	
CLIENT	PLUNGE POOL COMPANY PTY LTD.	ASSOCIATE CONSULTANT	

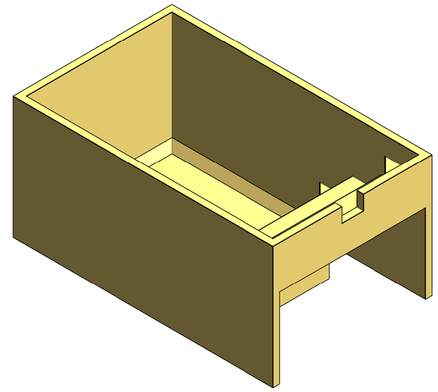
SCALES	
	AS INDICATED AT A1
JOB NO	2017.0077
DRAWING NUMBER	S300
REVISION	C1

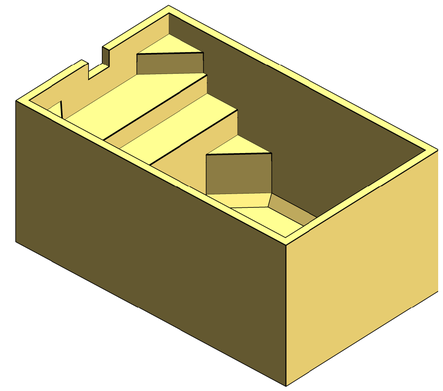
NOTE:
1. STRUCTURAL DRAWING TO BE READ IN CONJUNCTION WITH S300 GENERAL NOTES DRAWING, S301 GENERAL ARRANGEMENT PLAN, S302 REINFORCEMENT PLANS DRAWING AND S303 TYPICAL INSTALLATION DETAILS DRAWING.
2. WEIGHT OF SHELL = 5.7t APPROX. (EXCLUDING REINFORCEMENT AND FINISHES)
3. SKIMMER BOX SETDOWN AND CAST-IN CONDUITS SETOUT TO BE CONFIRMED PRIOR TO POUR. SKIMMER BOX AND CONDUITS REQUIRED ONE SIDE ONLY.



PRECAST CONCRETE SWIMMING POOL GENERAL ARRANGEMENT PLAN

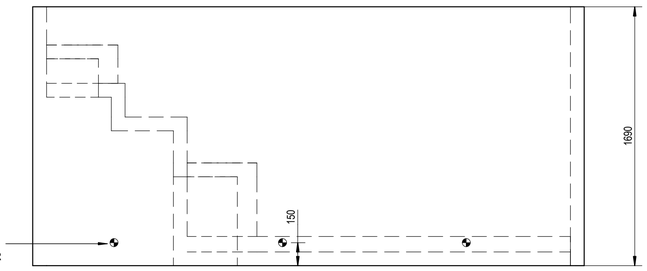
SCALE N.T.S





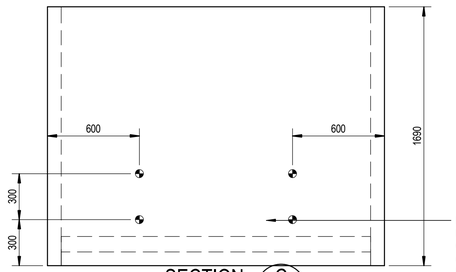
3D PERSPECTIVE VIEWS

CAST-IN LIFTING LUG INSERTS NOT SHOWN. REFER PLAN AND SECTIONS FOR LOCATIONS



DENOTES M20 x 70 LONG INSTALLATION LIFTING LUGS. REID ELEPHANT FOOT OR ANCON QWICKFOOT OR APPROVED EQUIVALENT. (STAINLESS STEEL) MAX WORKING SHEAR LOAD = 17kN (AT f'c ≥ 20 MPa) TO BE GROUTED UP AFTER INSTALLATION.

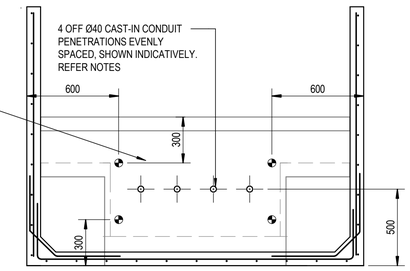
SECTION A S301
SCALE N.T.S



DENOTES M20 x 70 LONG INSTALLATION LIFTING LUGS. REID ELEPHANT FOOT OR ANCON QWICKFOOT OR APPROVED EQUIVALENT. (STAINLESS STEEL) MAX WORKING SHEAR LOAD = 17kN (AT f'c ≥ 20 MPa) TO BE GROUTED UP AFTER INSTALLATION.

SECTION C S301
SCALE N.T.S

DENOTES M20 x 70 LONG INSTALLATION LIFTING LUGS. REID ELEPHANT FOOT OR ANCON QWICKFOOT OR APPROVED EQUIVALENT. (STAINLESS STEEL) MAX WORKING SHEAR LOAD = 17kN (AT f'c ≥ 20 MPa) TO BE GROUTED UP AFTER INSTALLATION.



SECTION D S301
SCALE N.T.S

BLIGH TANNER

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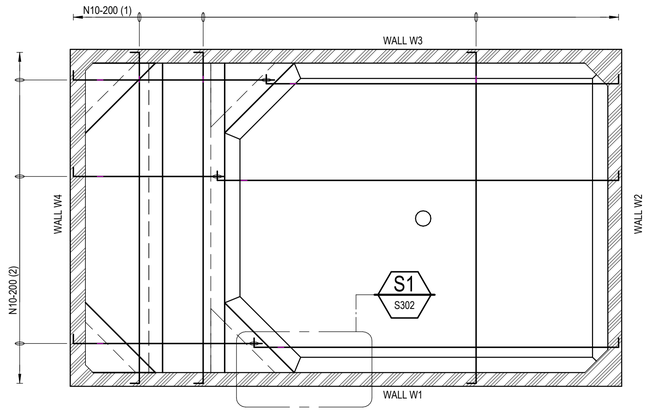
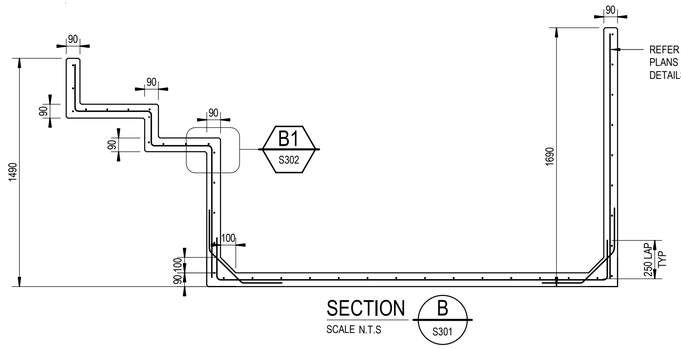
REV	DATE	DESCRIPTION	DESIGN	DRAWN	CHECKED	APPROVED	RPEQ No.
P1	17.07.2019	PRELIMINARY ISSUE	MW	RT			
C1	16.12.2019	CONSTRUCTION ISSUE	MW	MC			

PROJECT: PRECAST WATER HOLDING TANK	DRAWING TITLE: 3.6m x 2.2m SHELL GENERAL ARRANGEMENT PLAN AND SECTIONS	SCALES: AS INDICATED AT A1	
LOCATION	ARCHITECT	JOB NO: 2017.0077	
CLIENT: PLUNGE POOL COMPANY PTY LTD.	ASSOCIATE CONSULTANT	DRAWING NUMBER: S301	REVISION: C1



PRECAST CONCRETE SWIMMING POOL BASE REINFORCEMENT PLAN

SCALE N.T.S



SECTION B (S301)

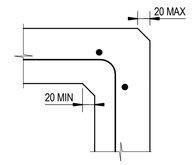
SCALE N.T.S

NOTES:

1. STRUCTURAL DRAWING TO BE READ IN CONJUNCTION WITH S300 GENERAL NOTES DRAWING, S301 GENERAL ARRANGEMENT PLAN, S302 REINFORCEMENT PLANS DRAWING AND S303 TYPICAL INSTALLATION DETAILS DRAWING.
2. SKIMMER BOX SETDOWN AND CAST-IN CONDUITS SETOUT TO BE CONFIRMED PRIOR TO POUR. SKIMMER BOX AND CONDUITS REQUIRED ONE SIDE ONLY.

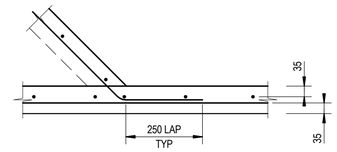
LEGEND

(1) ... DENOTES REINFORCEMENT LAID FIRST (AGAINST MOULD)
(2) ... DENOTES REINFORCEMENT LAID SECOND (EXTERNAL FACE)



SECTION B1 (S302)

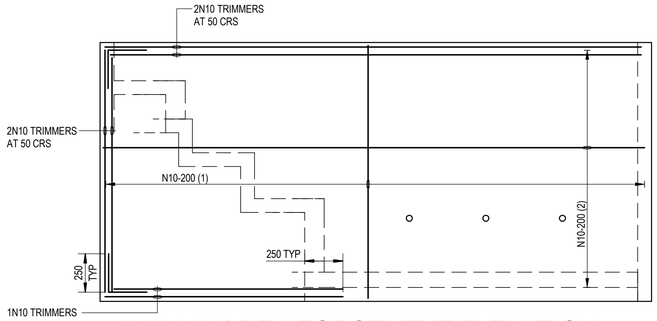
SCALE 1 : 5



TYPICAL INTERNAL TO EXTERNAL WALL DETAIL

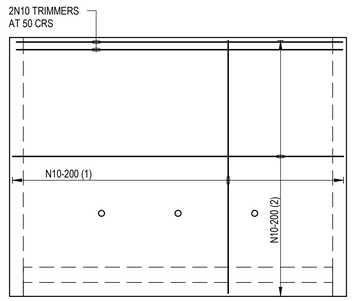
DETAIL S1 (S302)

SCALE 1 : 10



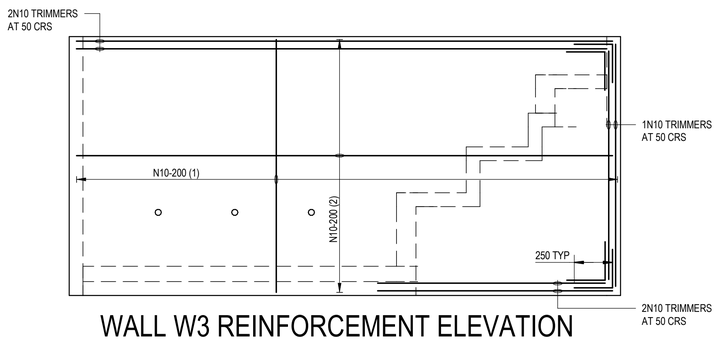
WALL W1 REINFORCEMENT ELEVATION

SCALE N.T.S



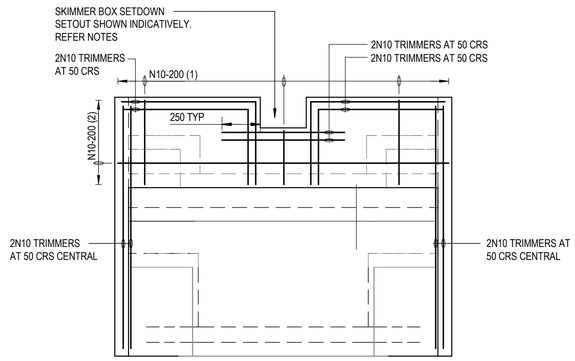
WALL W2 REINFORCEMENT ELEVATION

SCALE N.T.S



WALL W3 REINFORCEMENT ELEVATION

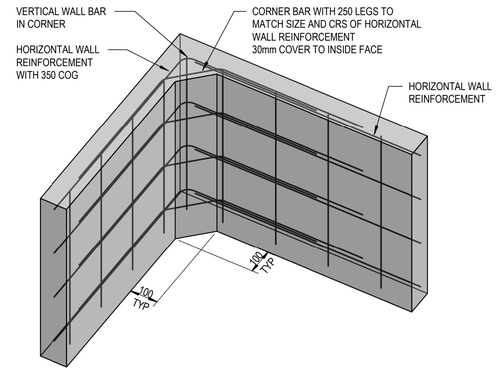
SCALE N.T.S



WALL W4 REINFORCEMENT ELEVATION

SCALE N.T.S

NOTE: REFER SECTION B FOR VERTICAL WALL REINFORCEMENT SETOUT AT STEPS.



TYPICAL WALL TO WALL MITRE CORNER DETAIL

WALL TO SLAB MITRE JOINT SIMILAR

50mm 0 50 100 150 200
1 :5

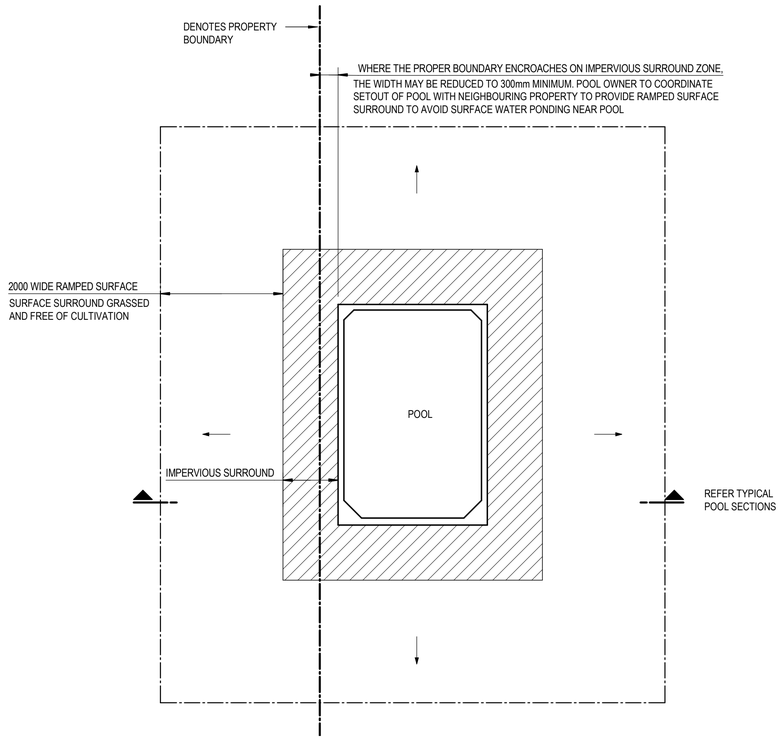
100mm 0 100 200 300 400
1 :10

BLIGH TANNER

LEVEL 9, 269 WICKHAM STREET, PO BOX 612
FORTITUDE VALLEY QLD 4006 AUSTRALIA
T 07 3251 8555 F 07 3251 8599

REV	DATE	DESCRIPTION	DESIGN	DRAWN	CHECKED	APPROVED	RPEQ No.
P1	17.07.2019	PRELIMINARY ISSUE	MW	RT			
C1	16.12.2019	CONSTRUCTION ISSUE	MW	MC			

PROJECT: PRECAST WATER HOLDING TANK	DRAWING TITLE: 3.6m x 2.2m SHELL REINFORCEMENT PLAN AND SECTIONS	SCALES: AS INDICATED AT A1	
LOCATION	ARCHITECT	JOB NO: 2017.0077	
CLIENT: PLUNGE POOL COMPANY PTY LTD.	ASSOCIATE CONSULTANT	DRAWING NUMBER: S302	REVISION: C1

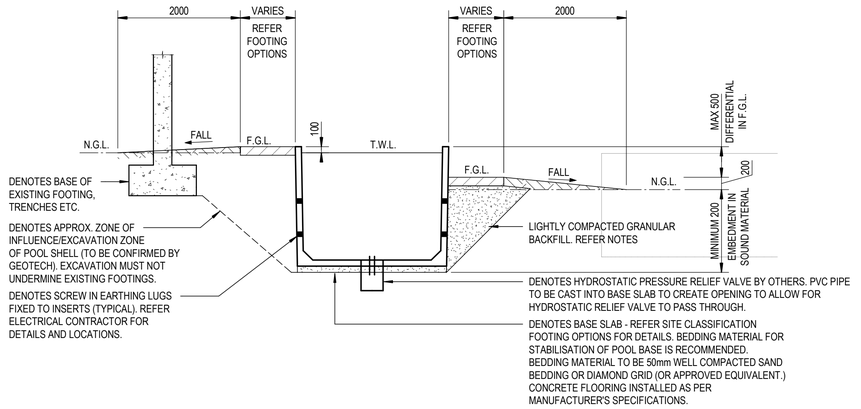


TYPICAL POOL PLAN

SCALE 1 : 50

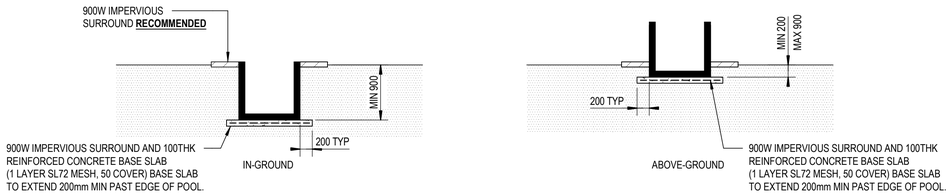
LEGEND:

T.W.L.	**TOP WATER LEVEL**
F.G.L.	**FINISHED GROUND LEVEL**
N.G.L.	**SOUND NATURAL GROUND LEVEL**

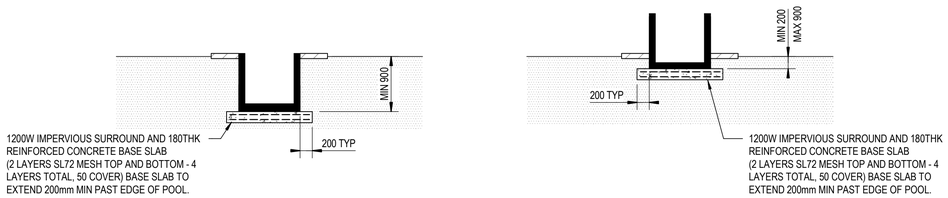


TYPICAL POOL SECTION

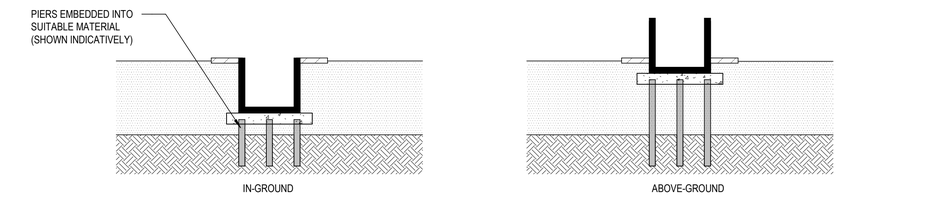
SCALE 1 : 50



SITE CLASS A/S/M



SITE CLASS H1/H2



SITE CLASS E - SITE SPECIFIC DESIGN REQUIRED

NOTE: DESIGN OF FOUNDATIONS FOR SITE CLASS E ARE SHOWN INDICATIVELY ONLY. FOUNDATIONS TO SUPPORT PRECAST SHELL IN SITE CLASS E ARE TO BE DESIGNED AND CERTIFIED BY A CERTIFIED ENGINEER.

BLIGH TANNER
LEVEL 9, 269 WICKHAM STREET, PO BOX 612
FORTITUDE VALLEY QLD 4006 AUSTRALIA
T 07 3251 8555 F 07 3251 8599

REV	DATE	DESCRIPTION	DESIGN	DRAWN	CHECKED	APPROVED	RPEQ No.
P1	17.07.2019	PRELIMINARY ISSUE	MW	RT			
C1	16.12.2019	CONSTRUCTION ISSUE	MW	MC			

PROJECT: PRECAST WATER HOLDING TANK	DRAWING TITLE: 3.6m x 2.2m SHELL SITE CLASSIFICATION FOOTING OPTIONS	SCALES: AS INDICATED AT A1	
LOCATION	ARCHITECT	JOB NO: 2017.0077	
CLIENT: PLUNGE POOL COMPANY PTY LTD.	ASSOCIATE CONSULTANT	DRAWING NUMBER: S303	REVISION: C1